

### 704-571-1323

### **Rack Post Load Design Calculation Details**

**Date:** 5/1/2014

Name: Sample ProjectLocation: Buffalo Grove

Engineer: Mike McPhee

Poisson's Ratio  $\mu =$ 0.15 Section Width 1 meter Slab Thickness h = 10.50 in Concrete Compressive Strength 4,000.00 psi Modulus of Elasticity E = 3,602,728.70 psi Concrete Flexural Strength 600.00 psi k = Modulus of Subgrade Reaction 100.00 pci Post Load P = 12,000.00 lb **Contact Area Dimensions** x = 6.00 in y = 6.00 in Load Spacing W = 42.00 in 96.00 in Z = 14.00 in

## **Radius of Relative Stiffness**

$$I = \begin{bmatrix} E * h^3 \\ \hline 12(1 - \mu^2) * k \end{bmatrix} 0.25$$

/ = 43.37 in

(1,101.66 mm)

## **Load Percentage**

Post System Configuration = Back to Back

<u>Main Rack</u>			<u>Adjacent Rac</u>	<u>K</u>			
Post	=	100.0%	Post <sub>7</sub>	=	40.3%		
Post <sub>W</sub>	=	12.9%	Post Z+L	=	0.4%	A =	23,091.42 lb
Post <sub>L</sub>	=	1.1%	Post <sub>ZW</sub>	=	11.7%		(10.31 tonne)
Post WL	=	0.7%	Post (Z+L)W	=	0.1%		

# Load Contact Radius - If Rack to Rack Distance(Sz) >= (2 \* h) then

$$a = \begin{bmatrix} x * y \\ \hline \Pi \end{bmatrix} 0.5$$

<u>else</u>

$$a = \begin{bmatrix} (Sw + x) * y \\ \hline \Pi \end{bmatrix}$$
 0.5  $a = 3.39 \text{ in}$ 

## If contact radius(a) < (1.72 \* h) then

$$b = \begin{bmatrix} 1.6 * a^2 + h^2 & 0.5 \\ & - (0.675 * h) \end{bmatrix}$$

<u>else</u>

$$b = a$$
  $b = 4.25 in$  (0.11 m)

## **Slab Stress - Interior Loading**

$$f_b = 2.70 * (1 + \mu) * (P / h^2) * (4.0 * log(R / b) + 1.069) * 10^6$$
  $f_b = 333.09 \text{ psi}$  (2,296.60 kN/m²)

# **Interior Safety Factor**

FS = 
$$\frac{f_r}{f_b}$$
 Corner = 1.48 Edge = 1.28 FS = **1.80**

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